Modelling non-stationary flood frequency in England and Wales using physical covariates

Short title: Physical covariates for non-stationary flood frequency

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Abstract

Non-stationary methods of flood frequency analysis are widespread in research but rarely implemented by practitioners. One reason may be that research papers on non-stationary statistical models tend to focus on model fitting rather than extracting

15 the sort of results needed by designers and decision makers. It can be difficult to extract useful results from non-stationary models that include stochastic covariates for which the value in any future year is unknown.

We explore the motivation for including such covariates, whether on their own or in addition to a covariate based on time. We set out a method for expressing the results of non-stationary models as an integrated flow estimate, which removes the dependence on the covariates. This can be defined either for a particular year or over a longer period of time. The methods are

20 illustrated by application to a set of 375 river gauges across England and Wales. We find annual rainfall to be a useful covariate at many gauges, sometimes in conjunction with a time-based covariate.

For estimating flood frequency in future conditions, we advocate exploring hybrid approaches that combine the best attributes of non-stationary statistical models and simulation models that can represent changes in climate and river catchments.



Key words: England, flood frequency, non-stationary, physical covariate, Wales.

30 Highlights:

- 1. We explore why and how to include physical variables as covariates in statistical models of flood frequency.
- We develop and illustrate methods for extracting flow estimates from such models so that practitioners can obtain useful results.
- 3. Practitioners now have tools and guidance to apply non-stationary methods for flood management in England.

35 **Introduction**

As the impacts of environmental change become increasingly evident in time series, non-stationary methods of frequency analysis have become widespread in research. Examples include analysis of rainfall frequency, extreme sea levels and fluvial flood frequency, the primary focus of this paper.

Application of non-stationary methods by practitioners involved in engineering and environmental management is much more limited. We surveyed practitioners and flood management authorities in seven countries, asking whether their currently recommended methods of flood and/or rainfall frequency estimation accounted for non-stationarity over the period of measurements (Luxford and Faulkner, 2020). Apart from methods that account for sea level rise, non-stationary methods of flood frequency estimation were not mandated by flood management authorities or generally used by practitioners in the USA, Canada, Australia, the Netherlands, Germany or (until 2020) the UK. The only example that we found of them being used by

45 practitioners before 2020 was in Switzerland, where the Federal Office for the Environment has fitted a range of non-stationary models to peak flow data from many catchments (BAFU, 2017).

There are many possible reasons why the large body of research on non-stationary methods is not much implemented in practice. François et al. (2019) suggest these include the proliferation of approaches and the challenges of diagnosing and modelling drivers and effects of change. Here we focus on another barrier to implementation: the difficulty in extracting useful

50 results from some non-stationary statistical models of flood frequency. Research papers tend to focus on fitting non-stationary statistical models rather than extracting the sort of results needed by designers and decision makers (Schlef *et al.*, 2023). There is a particular difficulty in extracting results from models that use stochastic physical variables as covariates. This <u>difficulty</u> is well illustrated in a paper by Hesarkazzazi *et al.* (2021) who present results from non-stationary models of flood frequency in north-west England that show are conditional on the accumulated rainfall during the year. Results such as these cannot be directly used in flood risk management, because every year the rainfall will be different. The authors identified a need for further research into defining a frequency distribution for the covariates when introducing an extra stochastic component into

a model. Similar comments apply to papers by Šraj et al. (2016) and Chen, Papadikis et al. (2021).

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60 <u>The motivation for including physical covariates is worth exploring further.</u> The most common approach to non-stationary flood frequency estimation is to model the non-stationarity only as a function of time (Hesarkazzazi *et al.*, 2021). However, many authors include physical quantities as covariates, such as annual rainfall (Sraj, 2016; Yan *et al.*, 2017; Hesarkazzazi *et al.*, 2021; Chen, Papadikis *et al.*, 2021), extreme rainfall (Prosdocimi *et al.*, 2014), temperature (Hesarkazzazi *et al.*, 2021; Wasko, 2021), urban extent (Prosdocimi *et al.*, 2015) or climatic indices such as the North Atlantic Oscillation (NAO) or East

65 Atlantic pattern (EA) (Steirou et al., 2019).

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One of two reasons is typically given for using physically-based covariates:

- A. They help remove some of the year-to-year variability in maximum flows, enabling better identification of time-based trends and better fit of the distribution (Prosdocimi *et al.*, 2014; Hesarkazzazi *et al.*, 2021).
- B. They provide a more physically meaningful model of non-stationarity, since time of itself has no physical influence
- on flooding (Chen, Papadikis *et al.*, 2021). As a covariate, time is merely a substitute for some physical quantity that is influencing floods. Some physical covariates may open up the prospect of predicting the future evolution of the flood frequency curve (Sraj *et al.*, 2016).

Reason A leads to models that include both time and physical quantities as covariates. One important consideration is the effect of collinearity and confounding variables on results. The greater the dependence between covariates then the harder it is to

75 interpret the regression coefficients. When time is one of the covariates, collinearity can be avoided by detrending the physical covariates before inclusion in the statistical model. The time covariate will then represent the presence of any temporal trend in the flood peak series.

Reason B tends to lead to an approach in which the physical variables replace time as a covariate. To model temporal trends in flood series it is then necessary to include at least one physical covariate in the model which exhibits a time trend. If this

80 approach is to be used to understand future changes in flood risk, there is a need to model the trend in that covariate. The hope is that a covariate can be found for which the trend is easier to model than that in the flood series, perhaps because it has less variability and more predictability into the future. An example might be the extent of urbanisation in a catchment, which can be typically expected to show a monotonic increase over time and can be reasonably predicted into the future under a range of scenarios.

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A risk associated with the application of this second approach is the confusion of correlation for causation. As discussed in the next section, even apparently plausible physical covariates do not necessarily give a full explanation of trends. This can lead to a false sense of confidence about our ability to estimate the future evolution of the flood frequency curve: we might end up with a covariate for which we can confidently predict future values, but which is no more useful than the date as a way of

95 explaining observed trends in flood magnitudes.

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This potential for misinterpretation has led some authors to criticise the move to non-stationary frequency modelling, or to express caution. The arguments are summarised in Faulkner *et al.* (2019).

There is also a need to quantify the uncertainty associated with the results of non-stationary models, which can be expected to be larger than the uncertainty of stationary estimates of flood frequency (Serinaldi and Kilsby, 2015).

- Another challenge in implementing non-stationary models in practice is the difficulty of representing future conditions. Schlef et al. (2020) provide a classification of approaches for projecting future flood hazards, including trend-informed, climateinformed and hydrological simulation. Trend-informed refers to non-stationary statistical models with time as the only covariate. It is difficult to justify using such models to predict future trends. Both the climate-informed and hydrological simulation approaches make use of projections from physics-based climate models. The climate-informed approach uses these
- 105 projections in non-stationary statistical models with climatic covariates. Hydrological simulation is the conventional approach using rainfall-runoff modelling with projected climate inputs, typically carrying out flood frequency analysis for quasistationary subdivisions of the climate model output (Schlef *et al.*, 2023).

One argument in favour of the climate-informed approach is that it can use climatic variables related to large-scale oceanicatmospheric patterns, such as mean annual temperature or rainfall, that global and regional climate models can predict more

110 accurately than local short-duration rainfall intensity (e.g. Sraj *et al.*, 2016, Schlef *et al.*, 2020). Wasko (2021) suggests estimating future flood frequency using non-stationary models with climatic covariates projected using global climate models and the covariates being attributed to observed changes. One such suggested covariate is the product of monthly temperature and monthly rainfall (Towler *et al.*, 2010).

On the other hand, even with physically plausible covariates, climate-informed statistical models cannot distinguish correlation from causation (Slater *et al.*, 2021a). Such covariates may not necessarily be the sole or main driving mechanism for floods in

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a particular catchment, but if the covariate is increasing along with the floods, there may be a correlation. For example, although climate change is expected to affect annual rainfall, a common covariate, it can also be expected to influence other factors that

120 control flood magnitudes such as storm tracks, rainfall intensity and evapotranspiration (which influences soil moisture). These effects cannot all easily be represented by the relatively simple covariates used in the climate-informed approach. Hence, it is necessary to demonstrate a mechanistic link between the covariate(s) and flood magnitudes (Wasko *et al.*, 2021). Some similar arguments can be made against the current generation of the hydrological simulation approach. For example, the

effects of climate change on storm intensity are not represented in the hydrological simulations that were used to derive current

The hydrological simulation approach can lead to derivation of change factors which practitioners can apply to estimates of

- 125 guidance on the impacts of climate change on peak river flows in England. Kay *et al.* (2021) used changes in monthly rainfall and potential evaporation, running simulation models at a daily time step and assuming that daily rainfall scales with the change in monthly rainfall. Since most catchments in England have a critical storm duration of hours up to a few days, changes in monthly rainfall may not be well representative of the intensification of flood-producing rainfall on most rivers.
- 130 flood frequency made for a baseline period. For example, in England, change factors vary with river basin, with epoch (2020s, 2050s or 2080s) and are provided for a range of percentiles which express some of the uncertainty in the climate projections (Environment Agency, 2021). One difficulty with this approach is that it tends to assume a stationary baseline period, i.e. climate change is treated as a purely future phenomenon. This assumption is difficult to reconcile with the provision of change factors that show (mainly) increases in peak flows for the present epoch, the 2020s, in comparison with an earlier baseline
- 135 period.

In the discussion (section 6) we make some suggestions for reconciling the two approaches to estimating future flood frequency.

In the Methods section we first outline the types of non-stationary models of flood frequency and how they were fitted. We then set out ways of applying such models that can provide the type of information needed by practitioners. We illustrate these

140 new approaches with an application in England and Wales. The remaining sections present the results from this application, discuss our findings, draw conclusions and point to future steps. **Deleted:** Paper structure and background

The research originated within a project funded by the Environment Agency in England that produced methods, tools and guidance to equip practitioners to carry out non-stationary analysis of fluvial flood frequency (Faulkner *et al.*, 2020), although

145 the analysis and results presented here include additional model types and findings.

Methods

Non-stationary flood frequency models including physical covariates

The method described here is based on fitting frequency distributions to annual maximum river flows. All results shown here are for the Generalised Extreme Value (GEV) distribution. It is fitted using maximum likelihood estimation (MLE).

150 The GEV distribution function is of the form

$$F(x) = exp\left\{-\left[1+\xi\left(\frac{x-\mu}{\sigma}\right)\right]_{+}^{-1/\xi}\right\}$$
(1)

where μ is the location parameter, σ is the scale parameter, ξ is the shape parameter, $\xi \neq 0$ and $\sigma > 0$.

For the sake of simplicity, it was assumed that μ and log σ vary linearly with covariates, with ξ modelled as a constant. For a vector of covariates x:

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$$\mu_{(x)} = \mu_0 + \mu_1 x_1 + \mu_2 x_2 + \cdots$$
 (2)
 $\sigma_{(x)} = \exp(\phi_0 + \phi_1 x_1 + \phi_2 x_2 + \cdots).$ (3)

Thus for a non-stationary fit there are two or more elements of the location parameter to estimate, a constant component μ_0 and μ_1 , μ_2 , etc. which represent the influence of the covariates on the parameter, and the same for the scale parameter. Physical covariates were included in candidate non-stationary models in accordance with the following:

- Up to two covariates per model, with a maximum of one being a physical covariate. Collinearity between the physical covariates was a reason to avoid combining them, as was a need to keep the number of candidate models manageable.
 For other applications in which covariates describing catchment land cover are combined with climatic covariates, it may be reasonable to allow combinations of physical covariates.
 - Covariates were considered for modelling either or both of the location and scale parameters.
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• Models with different physical covariates for the location and scale parameters were not allowed, given the theoretical way these two parameters link to the properties of the underlying distribution of the hourly river flow data.

The candidate models included:

(1) a stationary version,

(2) versions with just time as a covariate,

170 (3a) versions with just one detrended physical covariate,

(3b) versions with just one physical covariate as measured (without detrending), and

(4) versions with both time and a linearly detrended physical variable.

At gauges where version 3a fits best, it can be inferred that flood magnitude is statistically associated with the value of the (detrended) physical covariate, without any temporal non-stationarity. This can be regarded as a stepping stone to aid

- 175 understanding, rather than an end in itself, because there is no need to detrend the physical covariate if it is not being combined with time as a covariate. It would seem odd if year-to-year variations were captured by a detrended physical covariate without the longer-term changes in that covariate (before detrending) also being important. The value of version (3a) is that it can help to disentangle the effect of any long-term trend from shorter-term cycles or fluctuations in the physical covariates. Comparing the various models helps us to assess if it is the specifics of the physical covariate or simply that it is maybe approximately
- 180 linear over time which is causing the physical covariate to appear statistically significant. For example, if version (3b) performs better than (4) then this may indicate that the same physical covariate is capturing both inter-annual variations and a longerterm trend.

The best-fitting model was initially judged by the lowest Bayesian Information Criterion (BIC). Before results were extracted from the models, any gauges where a version of model 3a had the lowest BIC were reallocated with the version 3b model that had the lowest BIC.

Extracting results from non-stationary flood frequency models

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Concepts such as return period or annual exceedance probability (AEP) become unwieldy in a non-stationary setting, and various alternative definitions and measures of probability have been proposed. We use the concept of the **encounter**

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- probability, which is the probability of an event occurring over a defined number of years, such as the expected design life of a flood defence scheme. It is equivalent to the design life level, proposed by Rootzén and Katz (2013).
 Flood frequency estimates from a non-stationary distribution may change over time and may also depend on any physical covariates. For instance, if the only covariate is annual rainfall, then the flow with a specified annual exceedance probability (AEP) given 1000mm of rainfall is the flow expected to occur with that AEP under the (clearly hypothetical) conditions that
- 195 the annual rainfall is always 1000mm. We refer to this quantity as the conditional flow estimate. The conditional flow estimate may be useful when examining the probability of past floods, but it is less informative for design and planning for present-day or future floods. We introduce the term integrated flow estimate, which removes the dependence

marginal return level to flood frequency analysis, following a suggestion by Faulkner et al. (2019).

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on a particular value of the covariates. It is defined as the return level corresponding to the encounter probability averaged over covariates in a period of interest. The calculation method is set out below. The concept was introduced by Eastoe and 200 Tawn (2009), who refer to it as the marginal return level. This paper describes the first application of the concept of the

This section sets out methods for calculating both conditional and integrated flow estimates, in each case for a non-exceedance probability value of *p*.

Let $F(y|x; \theta)$ be the conditional distribution function of annual maximum flow, where *x* is a vector of covariate values for the year in which the annual maximum is considered, and θ is the vector of parameters of the distribution, including the coefficients that relate the distribution parameters to the covariates.

Let $y_p(x)$ be the conditional flow estimate for probability p (the conditional pth quantile), i.e. conditional on a particular set of values x for the covariates.

Let $F_{(y;\theta)}$ be the distribution function for annual maximum flow in the period of interest (e.g. current or future time window typically exceeding a year in duration – such as over a design lifetime), where θ is the vector of parameters of the distribution. This distribution function does not depend on the covariate values of the particular year in which the annual maximum occurs but incorporates the distribution of the covariates over the period of interest.

Let y_p be the integrated flow estimate for probability p (the marginal pth quantile).

Deleted: For instance, if the covariate is annual rainfall, then the flow with a 1% AEP given 1000mm of rainfall is the expected flow under the (clearly hypothetical) conditions that the annual rainfall is always 1000mm.

Let f(x) be the joint density function of the covariates for the time period of interest, so this distribution could change

depending on whether the period of interest is the current or a future time window.

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Let Φ be the domain of the covariates.

 $F_{(y;\theta)}$ can be obtained from the conditional distribution function $F_{(y|x;\theta)}$ by integrating out the covariates:

$$F(y;\theta) = \int_{\Phi} F(y|x;\theta) f(x) dx.$$
(4)

The integrated flow estimate for the period of interest is then y_p , which is such that $F(y_p; \theta) = p$.

The integrated flow estimate for a year in the period of interest is obtained by inverting the distribution function: $y_p =$

225 $F^{-1}(p; \theta)$. This needs to be solved numerically.

In practice, the parameters θ of this distribution are not known, nor is the true density f(x) of the covariates, so to get an integrated flow estimate, they need to be replaced with estimates in Equation 4. Here, θ is estimated by θ , the maximum likelihood estimate of the parameters, and the joint density of the covariates f(x) needs to be replaced by some estimate f(x), for example, the empirical density of the data or a kernel density estimate, which smooths the empirical estimate (Silverman,

230 1998). The data used to construct this kernel density estimate depends on the period of interest.

The integrated flow estimate for probability p, y_p , is found from $y_p = F^{-1}(p; \theta)$.

Similarly, the conditional flow estimate for probability p is obtained from $y_p(x) = F^{-1}(p|x;\theta)$.

The integrated flow estimate should be understood as applying over a period rather than instantaneously. This is a useful concept for planning investment decisions in flood risk management.

- 235 If the covariates include both time and physical variables, it is possible to calculate an integrated flow estimate by averaging the probabilities corresponding to the observed physical covariate values, but setting the time covariate to a single value, such as the final year of record. This gives what we term a **single-year integrated flow estimate**. If the river flow record runs up to the present day, this estimate is representative of the flow expected to be exceeded with a given probability under current conditions, without being conditional on any particular value of a covariate. The single-year integrated flow estimate can be
- 240 more easily compared with alternative estimates such as those from a model that uses only time as a covariate. A method for calculating confidence limits of the estimates is presented in Faulkner *et al.* (2020).
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Application in England and Wales

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	Dataset and screening	Formatted: Font: Bold
	The method described above was applied to annual maximum flow series at a set of 375 river gauging stations in England and	
245	Wales. The dataset was screened to eliminate gauges where apparent non-stationarity might arise from changes in the flow	
	measurement method or the local hydraulics. Details of the screening and of the final dataset are given in Faulkner et al. (2020),	
	which also presents the findings of non-parametric testing for trends and change points. These are not included here since our	
	focus is on parametric models of flood frequency and in particular the role of physical covariates.	
	The range of record lengths was from 27 to 134 years, with a median of 48 years. Data used for model fitting were restricted	Deleted: (Faulkner et al., 2020).
250	to the period 1950 onwards, the start year of the atmospheric circulation index datasets used.	
	Choice of physical covariates	Deleted: All five versions of the non-stationary GEV models described earlier were fitted at each gauging station, with every
		possible combination of the covariates. This resulted in 109 candidate models for each gauging station, which were then compared. The
	The following seven variables were selected as trial physical covariates:	calculations used the R package nonstat (Warren and Longfield, 2020)
	• Catchment-average rainfall, calculated over the water year (October to September), the autumn and the winter	
	seasons.	
255	• North Atlantic Oscillation (NAO) index, averaged over the winter, summer and autumn.	
	• East Atlantic pattern (EA) index, averaged over the winter.	
	Although some studies have used indices of extreme rainfall as covariates (e.g. Chen, Papadikis et al., 2021), we excluded	
	them to avoid shifting the problem into estimating the frequency distribution of an extreme value elsewhere in the hydrological	
	cycle. We also considered only covariates that are expected to be significant across many catchments in preference to those	
260	that represent locally specific effects such as urbanisation or changes in forest cover.	
	Rainfall accumulations were calculated from the CEH-GEAR dataset which provides daily rainfall on a 1km grid across the	
	UK from 1890 (Tanguy et al., 2016). NAO and EA indices were obtained from NOAA (2019). Covariates were centred and	
	scaled, subtracting the mean from each observation and dividing the result by the standard deviation.	



Trends in physical covariates

All covariates were significance tested for trend using the Mann-Kendall trend test, which is recommended for trend detection by the World Meteorological Organisation (2009). It is a non-parametric test which is based on the ranks of the data. A full description is given by World Meteorological Organisation (2009). The test was applied over the specific period of record of each individual gauging station. Table 1 summarises the significance of the trends. A large majority of stations show no evidence of significant trends in annual, autumn or winter rainfall over their catchment, at the 5% significance level. At 13% of stations, mostly in the north of England, there is a positive trend in annual rainfall, at the 5% significance level. Trends in the climatic indices NAO and EA depend entirely on the period of record covered by the gauging station, since the magnitude of these indices does not vary spatially. A positive trend in winter NAO is common for gauges with records starting in the 1950s-60s. Most stations apart from those starting before 1955 correspond with a period of (weaker) negative trend in the summer NAO.

Table 1: Summary of covariate trend test results at the 5% significance level

<u>Covariate</u>	% of stations with null hypothesis (not trend) not rejected	% with null hypothesis rejected and positive trend	% with null hypothesis rejected and negative trend
Water year rainfall	<u>87%</u>	<u>13%</u>	<u>0%</u>
Autumn rainfall	<u>100%</u>	<u>0%</u>	<u>0%</u>
Winter rainfall	<u>93%</u>	<u>7%</u>	<u>0%</u>
Winter NAO	43%	<u>57%</u>	<u>0%</u>
Summer NAO	<u>10%</u>	<u>0%</u>	<u>90%</u>
Autumn NAO	<u>100%</u>	<u>0%</u>	<u>0%</u>
Winter EA	25%	<u>75%</u>	<u>0%</u>

285 Model fitting

At each gauging station all five versions of the GEV models described in section 2 were fitted, i.e. one stationary model, one with time as a covariate and three versions with various implementations of physical covariates, some in conjunction with time. For these latter three, each of the seven physical covariates was tested. Covariates were applied to either or both of the location and scale parameters.

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Deleted: All covariates were tested for trend using the Mann-Kendall trend test over the specific period of record of each individual gauging station. Most stations show no evidence of significant trends in annual, autumn or winter rainfall over their catchment, at the 5% significance level.

295	This resulted in 109 candidate models for each gauging station, which were then compared. The calculations used the R* (Formatted: Line spacing: 1.5 lines
	package nonstat (Warren and Longfield, 2020).
	Results
	In presenting the results, we start by asking how much improvement in fit results from adding physical covariates to models
	versus modelling non-stationarity only as a function of time. We examine which model versions are preferred most often across
300	England and Wales and investigate any geographical patterns or relationships with catchment properties. We then move on to
	ask which physical covariates appear in the preferred models, and again whether there are geographical patterns in this aspect
	of the results. Finally we present flow estimates extracted from the preferred models, first at two example sites and then a
	comparison at the national scale between the preferred models and stationary models, including an assessment of confidence
	intervals, Formatted: Font: 10 pt
305	Preferred model versions
	Figure 1 compares the BIC of the best fitting non-stationary model at each station with the stationary model, and Table 2. Deleted: 1

records the proportion of gauges with the lowest BIC for each model type.



310 Figure 1: Boxplots showing the distribution of the BIC improvement between the best-fitting model of each version and the stationary model

Table 2: Proportions of model versions preferred across the dataset Model (1) (2) Non-(3a) Temporally (3b) Non-(4) Nonstationary: stationary: time version Stationary: stationary with stationary: No time the detrended physical measured and detrended physical covariates only covariates physical covariate (subsequently covariates covariates replaced with 3b)

26%

2%

% of gauges

1%

315 The results show that physical covariates (versions 3 and 4) give an improvement in fit over the stationary model at 97% of gauges. A large majority of these (74% of all gauges) were version (3): models without time as a covariate. In contrast, at most gauges, version (2) models with time as the only covariate give a slightly worse fit than the stationary model. There are some exceptions where version (2) models give a substantial improvement in fit. It is clear that physical covariates are adding useful information in nearly all cases. The increase in model complexity is outweighed by the increase in goodness of fit.

48%

24%

- 320 These findings indicate that regressing against covariates which aim to pick up only linear trends ignores more statistically important additional sources of inter-annual variation. Even when linear trends are removed from covariates (model 3a), there is a near-universal improvement in model fit compared with the stationary model. This is an important finding which hints at a causal relationship between the physical covariates and flood flows.
- Figure 1 shows little difference in the fit of model versions (3a), (3b) and (4), although (3b) more commonly has the lowest
 BIC (Table 1). If the selection of models is limited to only (3b) and (4), at 64% of gauges (3b) fits better than (4). This is as expected, indicating that the linear trends captured by the time covariate included in (4) can often be sufficiently represented by progressive changes in a physical covariate. (3b) has an advantage of simplicity over (4), which has more covariates. Out of versions (3a) and (3b), (3b) shows the better fit at 71% of gauges. Again, this is expected to be the case, given the tendency for most gauges to show some progressive trend in annual maximum flow, which (3b) is more likely to be able to safure than the de-trended physical covariate used in (3a).
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We examined the results for any differences in the physical characteristics of the catchments for the groups of gauges at which each model type fits best. Comparing versions 3(a or b) and 4, a two-sided Student's t-test found no significant differences, at a 5% significance level, in the mean of any of the characteristics tested: catchment area, mean gradient, mean annual rainfall,

335 soil type, urban extent, influence of lakes and extent of floodplains. It appears that there is no consistent physical difference between catchments for which non-stationarity can be modelled using the chosen physical covariates and those for which an additional unknown covariate (represented by water year) adds useful information to the model. Neither is there any clear difference in the locations of these two groups of catchments (Figure 2).

All the following discussion considers results only from the preferred model at each gauge. As mentioned earlier, gauges where 340 a version of model 3a had the lowest BIC were reallocated with the version 3b model that had the lowest BIC.

Preferred physical covariates

When physical covariates are selected, the annual rainfall is by some way the most common choice (Table <u>3</u>). It is included as a covariate for the location parameter at 66% of all gauges, and for the scale parameter at 22% of all gauges (the scale is modelled as constant, with no covariate, at most gauges). In some cases, annual rainfall is included alongside time as a

345 covariate, and in others it is the sole covariate.

Table 3: Most commonly selected covariates

Covariates for the location parameter			Covariates for the scale parameter			
Rank	Covariate	%	of	Rank	Covariate	% of
		gauge where chose	s n			gauges where chosen
1	Annual rain	66		1	None (i.e. parameter is constant)	62
2	Winter rain	22		2	Annual rain	22
3	Time	15		3	Time	13
4	Autumn rain	5		4	Winter rain	4
Note: Percentages in the columns can add up to more than 100, because models can include both time and a physical covariate together.						

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The second most useful physical covariate was the winter rainfall, chosen at 22% of gauges for the location parameter and 4%

for the scale parameter.



355 Figure 2: Preferred model version and type of covariate chosen at each gauge

Figure 2 shows that, while the annual rainfall is widespread as a preferred physical covariate across the country, the winter rainfall tends to be preferred in some parts of southern England. There appears to be some correspondence between these

gauging stations and the locations of chalk outcrops, which are concentrated in central southern England and along a band running from there north-east into East Anglia. The correspondence is consistent with the results on Figure 3(a) which shows that catchments with winter rainfall as a covariate tend to have a higher baseflow index, indicating a more groundwaterdominated flow regime. This makes sense physically: flood flows on groundwater-dominated catchments are expected to be strongly linked with the level of the water table, which is determined mainly by the volume of winter recharge. On chalk catchments, rainfall outside the winter recharge season may be lost due to evaporative demands and so have little impact on

365 flood flows.



Figure 3: Boxplots showing the distribution of (a) baseflow index and (b) catchment rainfall for catchments classified by the physical covariate included in the preferred model. Rain_DJF is winter rainfall, Rain_SON is autumn rainfall and Rain_WY is water year rainfall.

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There is also an association between choice of physical covariate and the average annual rainfall of the catchment (Figure 3(b)). For example, where covariates other than annual, winter or autumn rainfall are preferred, this tends to be on catchments with high rainfall.

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Role of time as a covariate

380 When the water year is included as a covariate, the sign of its regression coefficient indicates the effect that it is having on the estimated flows. In most cases (83%), the coefficient on the location parameter is positive, so the magnitudes of flood flows are modelled as increasing over time. For the scale parameter the picture is more mixed: the coefficient is positive in 58% of cases. At these stations, the variability of flood flows is modelled as increasing over time.

Example results from non-stationary models

385 Example results for two stations in north-east England are shown in Figure 4. It is common to present results of non-stationary models as a time series, but this is less appropriate for models with physical covariates. Instead the results are shown in the probability domain, as integrated flow estimates for a range of encounter probabilities evaluated over the length of each gauged record. The magnitudes of the recorded annual maximum floods are marked on the y axis.



of record and (b) the Bedburn Beck at Bedburn, with 58 years of record

The covariates for the preferred model at Leven Bridge (Fig. 4a) are water year and annual rainfall for the location parameter,
 with the scale parameter modelled as constant. There is an upward trend in the location parameter. The non-stationary flow estimates are distinctly higher than the stationary, especially for low probabilities. They also have much wider confidence limits. The highest flood on record, 125m³/s, is associated with an encounter probability over the 48-year record length of 38% according to the stationary results and just under 80% according to the non-stationary results.

The results at Bedburn (Fig. 4b) are quite different. The covariates here are water year and Autumn NAO for the location and
 water year for the scale, with a strong upward trend for both parameters. The physical covariate appears to add little to the quality of model fit, since the model with only water year achieves a similar BIC. For this catchment the non-stationarity is thought to be due mainly to changes in land cover, specifically afforestation and felling. For all except low probabilities, the non-stationary estimates are lower, and with narrower confidence limits, than the stationary. Findings of this type are to be anticipated as the inclusion of covariates reduces variation which the stationary model takes as implying that the data have a heavier tail (Carter and Challenor, 1981).

At both stations, the non-stationary estimates lie within the 90% confidence interval of the stationary results.

Results at a national scale

Widening the focus to a national scale, we compare the flow estimates from the preferred model at each gauge with those from the stationary model, for three specified annual exceedance probabilities: 50%, 10% and 1% (Figure 5). Where the preferred

410 model includes time as a covariate, the single-year integrated flow estimates has been extracted, for the most recent year in the period of record (the year 2016, at 93% of gauges) to provide a near present-day estimate. Where the preferred non-stationary model includes physical covariates only, the integrated flow estimate has been extracted for comparison.



the stationary model fall within the (typically wider) 90% confidence interval of the non-stationary estimates. This is the case at 82% of stations for AEP 50% and at 90% of stations for AEP 1%.

Discussion	Deleted: and conclusions
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Model fitting and selection

440 One reason why the non-stationary models occasionally give results much larger than the stationary may be poor estimation⁴⁴ of the shape parameter, particularly at sites with a shorter record. At all four stations where the non-stationary estimate at the 1% AEP is more than twice the stationary estimate, the shape parameter is modelled as non-stationary. Estimation of the shape parameter could be improved by adding a penalty to the likelihood function to constrain the range of shape parameters such as the "geophysical prior" of Martins and Stedinger, 2000. Although this was attempted and not found to be helpful, further research could look at improved implementations.

A possibility for future investigation would be to fit models using regularisation methods such as LASSO (least absolute shrinkage and selection operator) regression. These could be a promising way of fitting more complex models without having to try large numbers of combinations of covariates, and of avoiding difficulties with collinear covariates. However, LASSO could still have problems picking between causal and non-causal covariates when they are all strongly collinear.

450 For site-specific studies that lead to decisions about investment in flood management measures, it is important to take a more nuanced view of model selection, accounting for other types of information in addition to the BIC measure of model fit (François *et al.*, 2019). These include other statistical measures such as AIC and likelihood ratios, inspection of model fit, consistency of model form between locations, hydrological reasoning, confidence limits and sensibility comparisons between the flow estimates and the peak flow data (Faulkner *et al.*, 2020).

455 Looking to the future

Decision makers need information on future flood frequency. In theory, the integrated flow estimate could be calculated by averaging over a distribution of covariate values intended to represent future conditions. As discussed earlier, this climate-

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Moved down [2]: This research has made it possible to apply non-stationary flood frequency analysis incorporating physical covariates to practical problems in flood risk management.

We have shown that incorporating stochastic physical covariates improves the fit of flood frequency models for nearly all river gauging stations in England and Wales. Annual rainfall and winter rainfall are the most commonly selected covariates out of those tested. Importantly, we have found evidence of a causal relationship between the physical covariates and flood flows, shown by the fact that even detrended covariates give a near-universal improvement in model fit compared with the stationary model.⁶ At about a quarter of stations, the best-fitting model is a non-

At adout a quarter or statutins, the observining index is a horistationary version that had time as a covariate in addition to detrended physical covariates. By integrating over the distribution of the covariates it is possible to extract estimates from such models that define the flow for a given encounter probability over the period of record (the integrated flow estimate). It is also possible to calculate the integrated flow estimate by integrating over a sample of covariates that spans a period different from that covered by the river flow data. For instance, the record of the covariates might be longer, enabling a more confident estimate of the distribution of the covariates.

If the integration is only over the physical covariates, the estimate can define the flow for a given AEP in a given year, for example the most recent year of record (the single-year integrated flow estimate). This novel concept can be a useful quantity for flood risk managers or insurers who need an updated estimate of flood hazards that avoids having to assume stationarity. More generally, integration could be restricted to stochastic physical covariates, as opposed to any others whose behaviour is reasonably predictable such as urban cover.

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informed approach is only valid if the physical covariates provide a complete causal description of the non-stationarity in peak flows, which is unlikely to be the case.

It would be desirable to have a hybrid approach that could draw on the strengths of both the climate-informed and hydrological simulation approaches (Schlef *et al.*, 2020, 2023). One possibility would be to use the outputs of hydrological simulation as a

- 505 covariate in climate-informed statistical models, for example using soil moisture as a covariate (Tramblay *et al.*, 2014). A hybrid approach might seamlessly model both past and projected future non-stationarity, and account for the impact of climate change on localised, short-term rainfall intensity as well as on longer-term rainfall and soil moisture. It should also allow for the possibility of non-stationarity due to land use change.
- There are some advantages to fitting statistical models to the output of physics-based climate models rather than only to observations. These include more confidence in identifying causal relationships and the ability to separate forced and stochastic components of the signal (Vecchi *et al.*, 2011). Slater *et al.* (2021b) promote so-called hybrid statistical-dynamical modelling as a way of taking advantage of the ability of physical models to predict and explain large-scale phenomena and the strengths of non-stationary statistical models to estimate probabilities of extreme events.

Such a hybrid approach would depend on having a coupled climate-catchment model that was capable of accurately 515 reproducing the evolution of the statistical characteristics of floods over a period of time equivalent to typical gauged river flow record lengths, and into the future as well. To achieve this on many UK catchments the climate model would need to run at a high resolution to resolve local-scale atmospheric convection. The UKCP CPM (convection-permitting model) (Chen, Paschalis *et al.*, 2021) meets this requirement and is capable of running for time slices covering several decades. The statistical component of a hybrid model might benefit from including a covariate that functions as an indicator variable to distinguish

520 between observed (past) and modelled (future) data. Brown *et al.* (2014) applied this technique when developing a nonstationary statistical model fitted to both observed rainfall and modelled projections of future rainfall.

	Conclusions	Formatted: Font: 14 pt
525	This research has made it possible to apply non-stationary flood frequency analysis incorporating physical covariates to some practical problems in flood risk management, although other barriers remain to widespread application (c.f. François et al., 2019), including the need for a method that can be applied at ungauged locations. We have shown that incorporating stochastic physical covariates improves the fit of flood frequency models for nearly all river gauging stations in England and Wales. Annual rainfall and winter rainfall are the most commonly selected covariates out of those tested. The findings are consistent with those of Chen, Papadikis <i>et al.</i> (2021) that annual rainfall tends to be the best covariate in lowland areas of England, with the annual maximum daily rainfall being preferred in the rest of the UK where	Moved (insertion) [2] Deleted: practical problems in flood risk management.
530	catchments tend to be dominated by low permeability bedrock and so sensitive to high intensity rainfall.	
	Importantly for a model to be used in prediction (Wasko et al., 2021), we have found evidence of a causal relationship between	
	the physical covariates and flood flows, shown by the fact that even detrended covariates give a near-universal improvement	
	in model fit compared with the stationary model.	
	At about a quarter of stations, the best-fitting model is a non-stationary version that had time as a covariate in addition to	
535	detrended physical covariates. By integrating over the distribution of the covariates it is possible to extract estimates from such	
	models that define the flow for a given encounter probability over the period of record (the integrated flow estimate). It is also	
	possible to calculate the integrated flow estimate by integrating over a sample of covariates that spans a period different from	
	that covered by the river flow data. For instance, the record of the covariates might be longer, enabling a more confident	
	estimate of the distribution of the covariates.	
540	If the integration is only over the physical covariates, the estimate can define the flow for a given AEP in a given year, for	
	example the most recent year of record (the single-year integrated flow estimate). This novel concept can be a useful quantity	
	for flood risk managers or insurers who need an updated estimate of flood hazards that avoids having to assume stationarity.	
	More generally, integration could be restricted to stochastic physical covariates, as opposed to any others whose behaviour is	
	reasonably predictable such as urban cover.	
545	Although the best-fitting non-stationary models often give results similar to those of stationary models, in some cases their	
	results are substantially higher or (less often) lower, occasionally falling outside the 90% confidence interval of the stationary	
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estimates. Despite the typically wide confidence intervals associated with non-stationary models, occasionally the stationary estimate falls outside the 90% non-stationary confidence interval. It appears that, while uncertainty is certainly a major feature

of extreme value distributions, it does not always obscure the signal of non-stationarity (c.f. Serinaldi and Kilsby, 2015).

The methods described in this paper are now available to practitioners in the form of a user-friendly R package and guidance document, and have been used since 2020, alongside conventional stationary methods, in the planning and design of flood alleviation schemes in England.

Code availability

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555 The procedures for non-stationary flood frequency estimation are implemented in the R package *nonstat*, available from https://www.gov.uk/flood-and-coastal-erosion-risk-management-research-reports/development-of-interim-nationalguidance-on-non-stationary-fluvial-flood-frequency-estimation.

Data availability

The dataset of annual maximum flows for England and Wales was based on the National River Flow Archive dataset available from https://nrfa.ceh.ac.uk/peak-flow-dataset. The version of the dataset used for the analysis is available from the authors. The results of the national analysis are available from https://www.gov.uk/flood-and-coastal-erosion-risk-managementresearch-reports/development-of-interim-national-guidance-on-non-stationary-fluvial-flood-frequency-estimation.

Author attribution

SL commissioned and reviewed the research, DF led the research team, SW developed the code and JT advised on statistical
 aspects. DF prepared the manuscript with substantial contributions from all co-authors.
 The authors declare that they have no conflict of interest.

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